<u>Clearwater Passive Treatment System</u> SRI O&M TAG Project #64 Request #1 OSM PTS ID: PA-65

<u>Requesting Organization:</u> Altoona Water Authority <u>Requesting Organization Representative:</u> Amy Sipes <u>Municipality/County:</u> Altoona, Blair County <u>Dates of work performed:</u> 5/8/2019 to 7/11/2019

<u>Initial Request:</u> On 5/8/2019, while BioMost (BMI) was working on the nearby Squatter Falls system, the Altoona Water Authority (AWA) requested assistance for the Clearwater passive system. The AWA reported there was a problem with the effluent of the ALD and that water was bypassing the system. They also noted that the water bypassing the ALD was causing erosion of the access road.

Initial Site Visit, Observations, and Identified Needs: Work on this project was to be conducted in conjunction with Saint Francis University. An initial site visit was conducted on 5/9/19 with additional investigations conducted throughout the project. The system is comprised of an underground coal mine wet seal with an outlet pipe extending into an Anoxic Limestone Drain (ALD) followed by a wetland. The raw water is low pH (4.4 average), with low metals concentrations (Fe, Al, Mn average all below 1.0 mg/L), and low acidity (9 mg/L average). The use of an ALD appears to be have been appropriate based on the laboratory analyses, however, field-measured dissolved oxygen at the inlet to the ALD on 7/5/2019 was 8.63 mg/L. A review of available documents (now posted to www.datashed.org) indicate that the ALD is 20 ft wide x 5 ft high x 460 ft long. Using a bulk density of 1.35 tons/cubic yard results in an estimate of 2,300 tons of AASHTO #3 limestone being placed in the ALD.

The primary maintenance issue appears to be that the inlet end of the ALD is clogged, which was indicated by water leaking from the inlet pipe immediately upgradient of the ALD. Monitoring wells located along the length of the ALD showed that the piezometric surface of the water in the ALD was about three feet higher at the inlet end than at the midpoint and closer to the outlet. The water that was flowing through the ALD is highly alkaline (111 mg/L on 7/5/2019) but comprises a very small portion of the total flow. 5 gpm of about 50 gpm was exiting the ALD on 7/5/2019 with 45 gpm flowing out of the inlet pipe and through a road culvert installed to convey the bypass water.

There is limited construction room in the area of the mine discharge and the ALD design seemed to be a reasonable approach to accommodate site conditions that also include an access road used for the Spaghetti Hole passive treatment system north of the Clearwater system. However, persistent plugging issues indicate that the ALD is not a practical long-term solution.

<u>Work Completed</u>: A culvert pipe was installed underneath the access road to divert flow from the road surface and reduce future erosion. Testing on water quality and various elevations helped to assess options to improve site conditions. The raw water pipes were exposed and two test pits were dug to determine permeability of the ALD media. After assessment, a raw water infiltration structure was built as a temporary inlet to the ALD.

Recommendations & Future Considerations:

Using a buried ALD has proved problematic due to plugging at the inlet of the limestone-based component. The inlet of the ALD is about eight feet below the existing ground surface which makes maintenance difficult, especially considering the limited construction area which is bounded by the access road. Raw water characteristics for current design purposes are defined using the chemistry sampled at the inlet to the ALD

(Point CWRAW/ALD) with flow determined by adding flow measured at the bypass (point CWRAW/ALD) and the outlet of the ALD (point CWALD/WET) on dates when flow measurements were taken at both points on the same day. The number of flow measurements is limited (n=5) but results in a higher maximum and average flow than either the ALD (which is often plugged) or the CWRAW/ALD (where raw water bypasses the ALD) alone, which will help ensure that the proposed system is designed for the maximum expected flow.

The primary purpose of the Clearwater Passive System is to neutralize acidity and produce alkaline water. Metal removal is minimal due to the low concentrations of iron, aluminum, and manganese in the raw water (all below 1 mg/L based on monitoring conducted 2016 through 2021). In order to produce alkaline water and provide maintenance access, a unique approach is proposed. This will involve excavating the existing ALD to recover as much of the AASHTO #3 limestone as feasible. It is expected that the limestone in the lower 400 feet of the ALD will be clean and should be able to be recovered with minimal effort. For estimating purposes, it will be assumed that about two-thirds (67%) of the initial limestone placed in the lower 400 feet of the ALD will be recovered:

20 ft wide x 5 ft high x 400 ft long / 27 cy/cf x 1.35 t/cy (bulk density) x 0.67 (recovery) = 1,340 tons

The proposed system will be constructed in the footprint of the existing access road and will consist of a threecell horizontal flow limestone bed (HFLB). The HFLB will be constructed to function as both a treatment component and access road. Approximately 3,000 tons of limestone will be needed to provide at least 12 hours of retention time and sufficient limestone to neutralize 2-3 tons of acidity over a 25-year period. In order to ensure that the mine drainage has the best opportunity to flow through the limestone, a manifold inlet will be installed that has an overflow riser located within the treatment component that will allow water to continue to be treated while maintenance work can be scheduled. The cells will be constructed using a 30-foot width and 5foot depth that will provide 50% more cross sectional area to accommodate the 200+ gpm flow.

The existing access road has about three feet of elevation change over about 450 feet. It is proposed that the three HFLB cells be constructed with the bottoms of each of the three cells each at a different elevation to accommodate the road grade, but with the top choked with AASHTO #3 aggregate as needed to provide a driving surface that has constant gradual slope. It is anticipated that only the first cell will require periodic maintenance and the two downstream cells will rarely need to be cleaned.

The first (inlet end) section will be fed by an inlet manifold with multiple perforated inlet laterals and contain new R-4 riprap to help ensure flow enters the limestone, the subsequent (middle) cell will be filled with new AASHTO #1 limestone, and the final cell will be filled with the recovered AASHTO #3 aggregate.

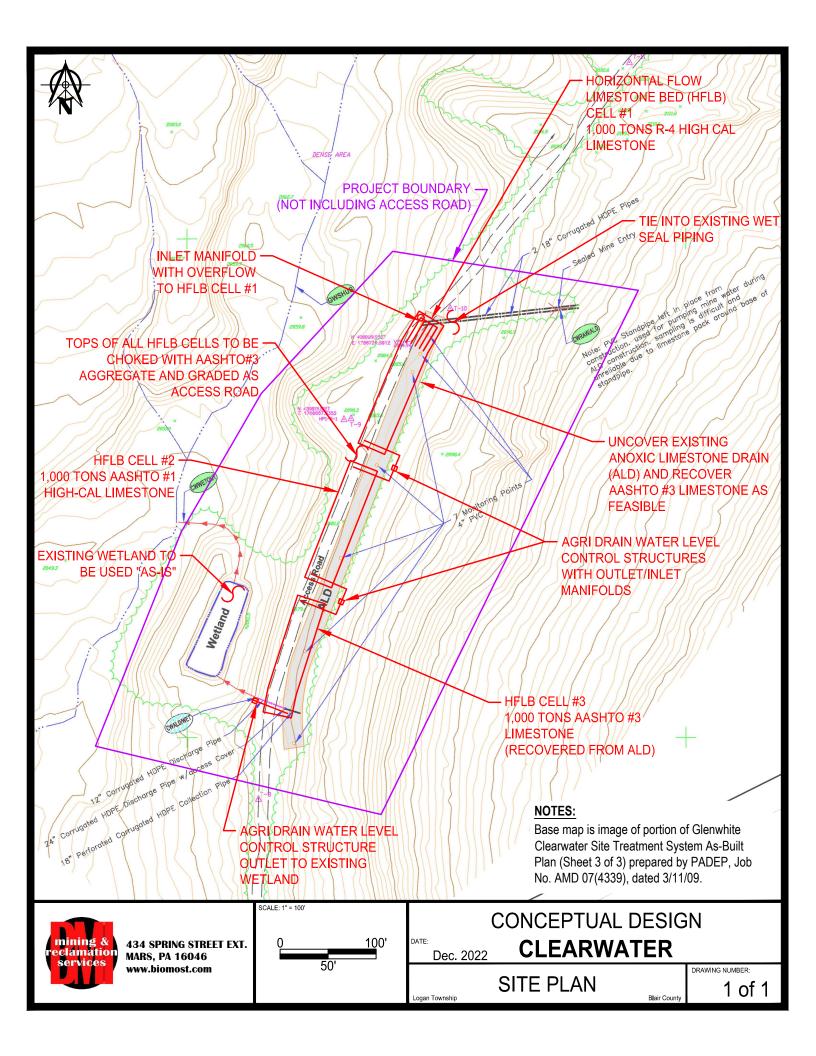
Each cell will have an outlet header pipe that will extend to an Agri Drain water level control structure. Cell two and three will be fed by an inlet header. Cell three will outlet via an outlet header and Agri Drain water level control structure to the existing wetland. Due to the low metal concentrations and frequent system bypass, no maintenance work is proposed for the existing wetland which will be used 'as-is'.

The inlet manifold should be constructed using HDPE pipe and assembled with rubber couplings (i.e., Ferncos) to allow the stone to be cleaned without damaging the inlet manifold piping. The expected maintenance would be triggered by water reaching the top of the overflow riser which would indicate that the manifold and/or aggregate around the manifold has become plugged. The stone would be excavated to expose the inlet manifolds and cleaned as needed. As the top of the stone will be at the road grade, the maximum excavation needed would be about six feet to the bottom of the limestone (as compared to the eight feet of excavation needed to reach the top of the ALD now with the bottom another five feet lower). It would be expected that the majority of the maintenance work will typically involve cleaning about 300 tons of limestone, which can be accomplished using a mini (3.5-5.0 ton) excavator in about a single day.

Photo Log



Top Left: ALD clogging led to short circuiting and flow on the access road.
Top Right: Limestone in the ALD was generally clean beyond the second monitoring well.
Bottom Left: A culvert was installed beneath the access road to act as a temporary overflow (bypass).
Bottom Right: A raw water infiltration structure was built as a temporary inlet to the ALD until system redesign.





Project <u>O&M TAG</u>

Site Name <u>Clearwater</u>



AMDTREAT

AMD TREAT AMD TREAT MAIN COST FORM

Costs	AN	ID T	REAT MAIN
Passive Treatment	<u>A</u>	<u>s</u>	
Vertical Flow Pond			\$0
Anoxic Limestone Drain			\$0
Anaerobic Wetlands			\$0
Aerobic Wetlands			\$0
Manganese Removal Bed			\$0
Oxic Limestone Channel			\$0
Limestone Bed	3	0	\$167,761
BIO Reactor			\$0
Passive Subtotal:			\$167,761
Active Treatment			
Caustic Soda			\$0
Hydrated Lime			\$0
Pebble Quick Lime			\$0
Ammonia			\$0
Oxidants			\$0
Soda Ash			\$0
Active Subtotal:			\$0
Ancillary Cost			
Ponds			\$0
Roads	1	0	\$18,459
Land Access			\$0
Ditching			\$0
Engineering Cost	1	0	\$42,944
Ancillary Subtotal:			\$61,403
Other Cost (Capital Cost)			\$28,500
Total Capital Cost:			\$257,664
Annual Costs			///////
Sampling			\$0
Labor			\$0
Maintenance			\$0
Pumping			\$0
Chemical Cost			\$0
Oxidant Chem Cost			\$0
Sludge Removal			\$0
Other Cost (Annual Cost)			\$0
Land Access (Annual Cost)			\$0
Total Annual Cost:			\$0
Other Cost	1	0	$\langle \rangle$

204.00	gpm
116.00	gpm
0.10	mg/L
0.10	mg/L
0.58	mg/L
0.24	mg/L
4.43	su
0.40	mg/L
10.53	mg/L
9.44	mg/L
65.51	mg/L
0.00	mg/L
9.72	С
0.00	uS/cm
0.00	mg/L
0.01	mg/L
2.3	tons/yr
	116.00 0.10 0.10 0.10 0.58 0.24 4.43 0.40 10.53 9.44 65.51 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

Total Annual Cost: per 1000 Gal of H2O Treated \$0.000

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COMMENTS:

Raw water characteristics are based on monitoring 1/11/2016 through 10/12/2021 downloaded from www.datashed.org (accessed December 2022).

Chemistry is raw (Point CWRAW/ALD) n=15

Flow measured at CWRAW/ALD and CWALD/WL (combined concurrent dates) n=5

Typical flow = Average

Design flow = Maximum

Limestone Bed (HFLB) liner intended to be geotextile liner on bottom and sides.

Access road cost includes geotextile to separate road stone from treatment limestone.

HFLB #3 using \$20/ton for limestone to account for estimated cost to expose and recover limestone in existing ALD.

Engineering Costs set at 25% (10% design, 10% permitting, 5% construction oversight)

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Site Name Clearwater



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AMD TREAT LIMESTONE BED (LSB) AMDTREAT

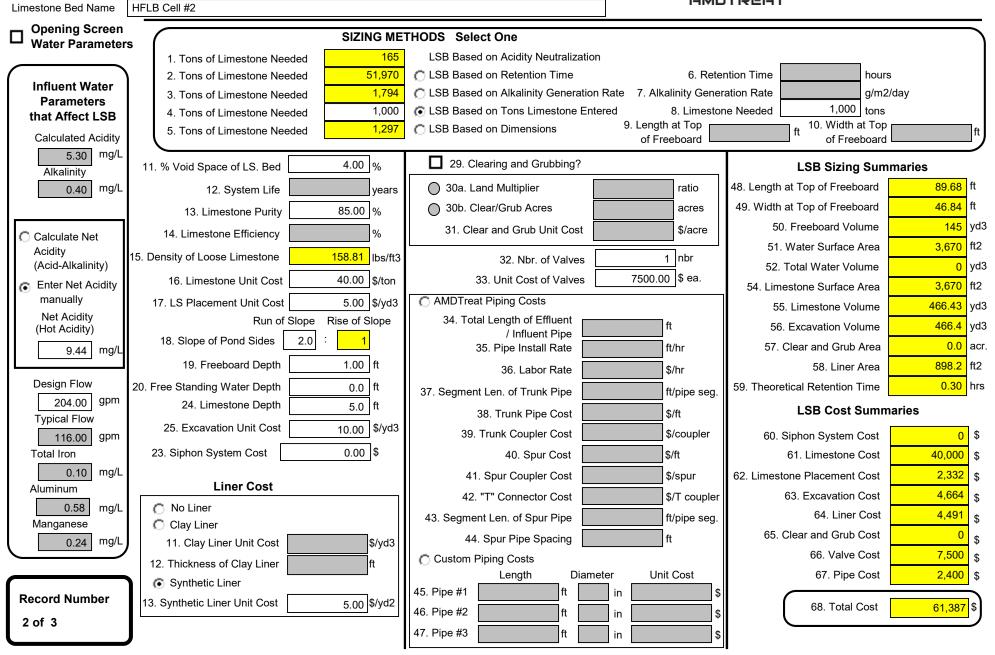
Limestone Bed Name	HFLB Cell #1	IHML	TREAT			
Opening Screen Water Parameter		SIZING ME	THODS Select One			
Water Farameter	1. Tons of Limestone Needed	165	LSB Based on Acidity Neutralization			
	2. Tons of Limestone Needed	51,970	C LSB Based on Retention Time	6. Reter	ntion Time hours	3
Influent Water Parameters	3. Tons of Limestone Needed	1,794	C LSB Based on Alkalinity Generation R	Rate 7. Alkalinity Gener	ÿ	/day
that Affect LSB	4. Tons of Limestone Needed		ISB Based on Tons Limestone Entered			
Calculated Acidity	5. Tons of Limestone Needed	1,297	C LSB Based on Dimensions	9. Length at Top of Freeboard	ft 10. Width at Top of Freeboard	ft
5.30 mg/L Alkalinity	11. % Void Space of LS. Bed	4.00 %	29. Clearing and Grubbing?		LSB Sizing Sum	maries
0.40 mg/L	12. System Life	years	30a. Land Multiplier	ratio	48. Length at Top of Freeboard	89.68 ft
	13. Limestone Purity	85.00 %	30b. Clear/Grub Acres	acres	49. Width at Top of Freeboard	46.84 ft
C Calculate Net	14. Limestone Efficiency	%	31. Clear and Grub Unit Cost	\$/acre	50. Freeboard Volume	145 yd3
Acidity	15. Density of Loose Limestone	158.81 lbs/ft3	32. Nbr. of Valves	1_nbr	51. Water Surface Area	3,670 ft2
(Acid-Alkalinity)	16. Limestone Unit Cost	40.00 \$/ton	33. Unit Cost of Valves		52. Total Water Volume	0 yd3
 Enter Net Acidity manually 	17. LS Placement Unit Cost	5.00 \$/yd3	AMDTreat Piping Costs		54. Limestone Surface Area	3,670 ft2
Net Acidity	Run of Slop		34. Total Length of Effluent		55. Limestone Volume	466.43 yd3
(Hot Acidity)	18. Slope of Pond Sides 2.0		/ Influent Pipe	ft	56. Excavation Volume	466.4 yd3
9.44 mg/L	19. Freeboard Depth	1.00 ft	35. Pipe Install Rate	ft/hr	57. Clear and Grub Area	0.0 acr. 898.2 ft2
Design Flow			36. Labor Rate	\$/hr	58. Liner Area	
204.00 gpm	20. Free Standing Water Depth 24. Limestone Depth	0.0 ft	37. Segment Len. of Trunk Pipe	ft/pipe seg.	59. Theoretical Retention Time	0.30 hrs
Typical Flow		5.0 ft	38. Trunk Pipe Cost	\$/ft	LSB Cost Summ	aries
116.00 gpm	25. Excavation Unit Cost	10.00 \$/yd3	39. Trunk Coupler Cost	\$/coupler	60. Siphon System Cost	0 \$
Total Iron	23. Siphon System Cost	0.00 \$	40. Spur Cost	\$/ft	61. Limestone Cost	40,000 \$
0.10 mg/L	Lines Orat		41. Spur Coupler Cost	\$/spur	62. Limestone Placement Cost	2,332 \$
Aluminum	Liner Cost		42. "T" Connector Cost	\$/T coupler	63. Excavation Cost	4,664 \$
0.58 mg/L Manganese	 No Liner Clay Liner 		43. Segment Len. of Spur Pipe	ft/pipe seg.	64. Liner Cost	4,491 _{\$}
0.24 mg/L	11. Clay Liner Unit Cost	\$/yd3	44. Spur Pipe Spacing	ft	65. Clear and Grub Cost	0 \$
0.24	12. Thickness of Clay Liner	φ, yασ	Custom Piping Costs		66. Valve Cost	7,500 \$
	Synthetic Liner	IL	Length Diamete	er Unit Cost	67. Pipe Cost	6,400 \$
Record Number	13. Synthetic Liner Unit Cost	5.00 \$/yd2	45. Pipe #1ft	in \$		
1 of 3		5.00 ^{φ, ydz}	46. Pipe #2ft	in\$	68. Total Cost	<mark>65,387</mark> \$
	J		47. Pipe #3 ft	in\$		

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Site Name Clearwater

HFLB Cell #3

Limestone Bed Name



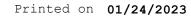
AMD TREAT

Opening Screen Water Parameters	SIZING METHODS Select One						
	1. Tons of Limestone Needed	s of Limestone Needed 165 LSB Based on Acidity Neutralization					
	2. Tons of Limestone Needed	C LSB Based on Retention Time	he 6. Retention Time hours				
Influent Water Parameters	3. Tons of Limestone Needed	1,794 C LSB Based on Alkalinity Generation Rate 7. Alkalinity C					
that Affect LSB	4. Tons of Limestone Needed	1,000	ISB Based on Tons Limestone Entered	8. Limestor			
Calculated Acidity	5. Tons of Limestone Needed	1,297		gth at Top Freeboard	ft 10. Width at Top		
5.30 mg/L							
Alkalinity	11. % Void Space of LS. Bed	4.00 %	29. Clearing and Grubbing?		LSB Sizing Sum		
0.40 mg/L	12. System Life	years	O 30a. Land Multiplier	ratio	48. Length at Top of Freeboard	89.68 ft	
	13. Limestone Purity	85.00 %	O 30b. Clear/Grub Acres	acres	49. Width at Top of Freeboard	46.84 ft	
C Calculate Net	14. Limestone Efficiency	%	31. Clear and Grub Unit Cost	\$/acre	50. Freeboard Volume	145 yd3	
Acidity	5. Density of Loose Limestone	158.81 lbs/ft3	32. Nbr. of Valves	1 nbr	51. Water Surface Area	3,670 ft2	
(Acid-Alkalinity)	16. Limestone Unit Cost	20.00 \$/ton).00 \$ ea.	52. Total Water Volume	0 yd3	
 Enter Net Acidity manually 				,.00 \	54. Limestone Surface Area	3,670 ft2	
Net Acidity	17. LS Placement Unit Cost	5.00 \$/yd3	C AMDTreat Piping Costs		55. Limestone Volume	466.43 yd3	
(Hot Acidity)	· · · ·	Rise of Slope	34. Total Length of Effluent / Influent Pipe	ft	56. Excavation Volume	466.4 yd3	
9.44 mg/L	18. Slope of Pond Sides 2.0	- <u>1</u>	35. Pipe Install Rate	ft/hr	57. Clear and Grub Area	0.0 acr.	
	19. Freeboard Depth	1.00 ft	36. Labor Rate	\$/hr	58. Liner Area	898.2 ft2	
	20. Free Standing Water Depth	0.0 ft	37. Segment Len. of Trunk Pipe	ft/pipe seg.	59. Theoretical Retention Time	0.30 hrs	
204.00 gpm Typical Flow	24. Limestone Depth	5.0 ft	38. Trunk Pipe Cost	\$/ft	LSB Cost Sumn	naries	
116.00 gpm	25. Excavation Unit Cost	10.00 \$/yd3	39. Trunk Coupler Cost	\$/coupler	60. Siphon System Cost	0 \$	
Total Iron	23. Siphon System Cost	0.00 \$	40. Spur Cost	\$/ft	61. Limestone Cost	20,000 \$	
0.10 mg/L			41. Spur Coupler Cost	\$/spur	62. Limestone Placement Cost	2,332 \$	
Aluminum	Liner Cost		42. "T" Connector Cost	\$/T coupler	63. Excavation Cost	<mark>4,664</mark> \$	
0.58 mg/L	No Liner		43. Segment Len. of Spur Pipe	ft/pipe seg.	64. Liner Cost	4,491 _{\$}	
Manganese	C Clay Liner 11. Clay Liner Unit Cost	\$/yd3	44. Spur Pipe Spacing	ft	65. Clear and Grub Cost	0	
0.24		φ/yu3	C Custom Piping Costs		66. Valve Cost	7,500 _{\$}	
	12. Thickness of Clay Liner	π		Jnit Cost	67. Pipe Cost	2,000 \$	
Record Number	Synthetic Liner		45. Pipe #1 ft in	\$			
	13. Synthetic Liner Unit Cost	5.00 \$/yd2	46. Pipe #2 ft in	\$	68. Total Cost	40,987 \$	
3 of 3			47. Pipe #3 ft in	\$			

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Site Name <u>Clearwater</u>



AMD TREAT

ROADS

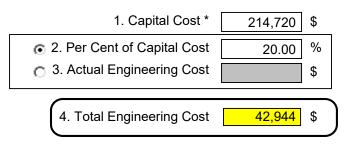


1. Road Length	460 ft	14. Reveg Unit Cost	3000.00	\$/acre
2. Road Width	12 ft	15. Culvert Unit Cost	50.00	\$/ft
3. Road Depth	1.00 ft	16. Culvert Length	200	ft
4. Aggregate Unit Cost	35.00 \$/yd3	Roads Sub-1	otals	
5. GeoTextile Length	460 ft	17. Road Surface Cost	7,156	\$
6. GeoTextile Unit Cost	2.00 \$/yd2	18. GeoTextile Cost	1,227	\$
7. Length of Silt Fence	0 ft	19. Silt Fence Cost	0	\$
8. Unit Cost of Silt Fence	0.00 \$/ft	20. Culvert Cost	10,000	\$
9. Surveying?	0.00	21. Revegetation Cost	76	\$
10. Survey Rate	acres/day	22. Survey Cost	0	\$
11. Survey Unit Cost	\$/day	23. Clear and Grub Cost	0	\$
□ 12. Clearing and Grubb		24. Total Cost	18,459	\$
13. Clear and Grub Cost	\$/acre			

Company Name <u>Stream Restoration Inc.</u> Project <u>O&M TAG</u>

Site Name <u>Clearwater</u>

AMD TREAT ENGINEERING COST



* Total Capital Cost minus Engineering and Land Access Capital Cost



Project O&M TAG

Site Name <u>Clearwater</u>

AMD TREAT

OTHER COST



AMOTREAT

Oher Cost Name Clearwater Impr					AMDTREF
A. Description of Item		B. Unit Cost Per Item	C. Quantity	D. Total Item Cost	E. Capital Cost Annual Cost
1. Tie into existing wet seal pi	ping (JOB)	5,000.00	1	5,000	 Capital Cost C Annual Cost
2. Mob/Demob (JOB)		9,500.00	1	9,500	 Capital Cost C Annual Cost
3. E&S Controls (JOB)		11,000.00	1	11,000	 Capital Cost C Annual Cost
4. Revegetation (AC)		3,000.00	1	3,000	 Capital Cost C Annual Cost
5.		0.00	0	0	 Capital Cost Annual Cost
6.		0.00	0	0	 Capital Cost C Annual Cost
7.		0.00	0	0	 Capital Cost C Annual Cost
8.		0.00	0	0	 Capital Cost C Annual Cost
9.		0.00	0	0	 Capital Cost C Annual Cost
10.		0.00	0	0	 Capital Cost C Annual Cost
11.		0.00	0	0	 Capital Cost C Annual Cost
12.		0.00	0	0	 Capital Cost C Annual Cost
13.		0.00	0	0	 Capital Cost C Annual Cost
14.		0.00	0	0	Capital Cost
15.		0.00	0	0	 Capital Cost C Annual Cost
Record Number 1 of 1	Curent Capit Current Annu			otal Capital Cos otal Annual Cos	st 28,500 \$